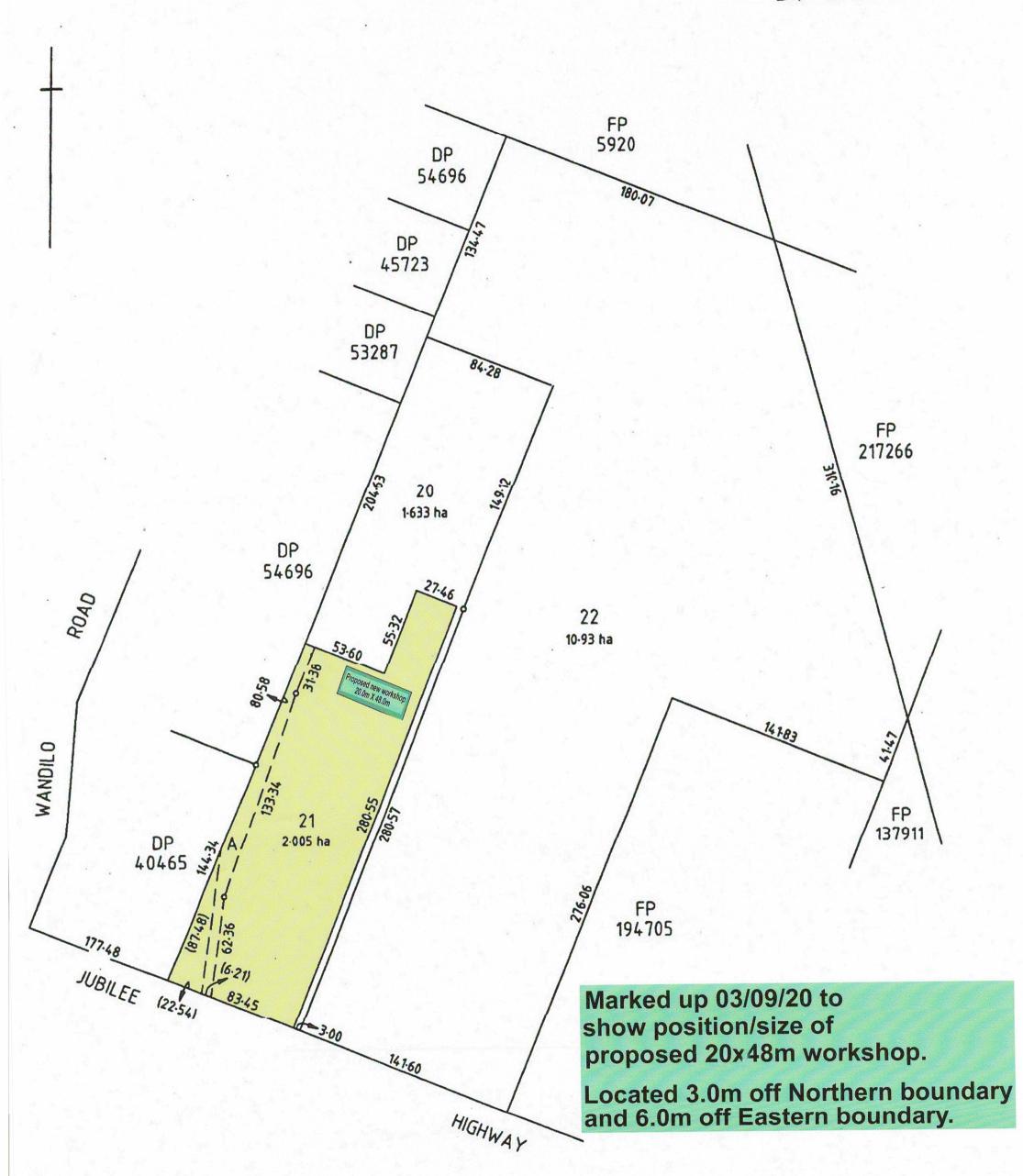
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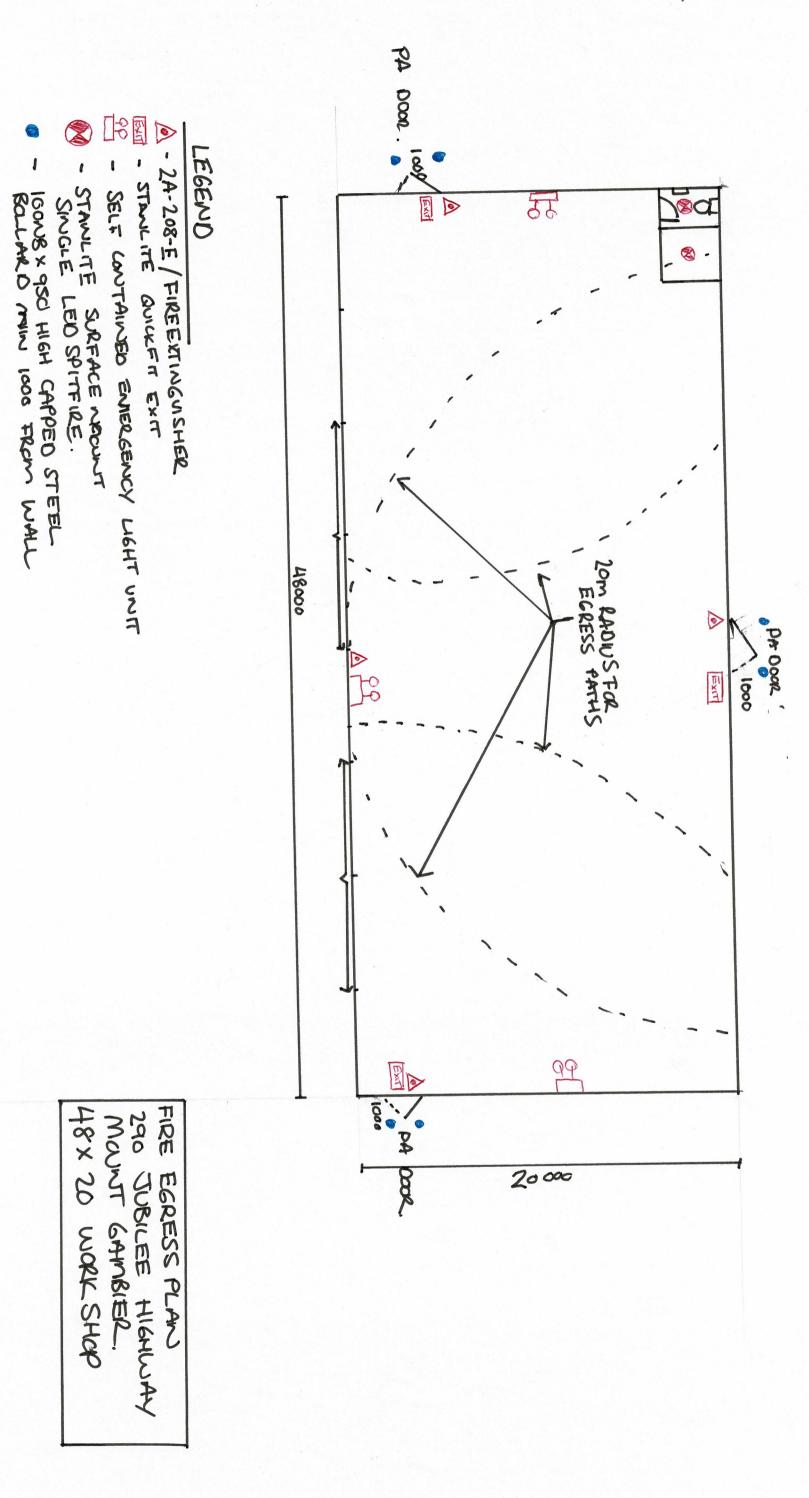
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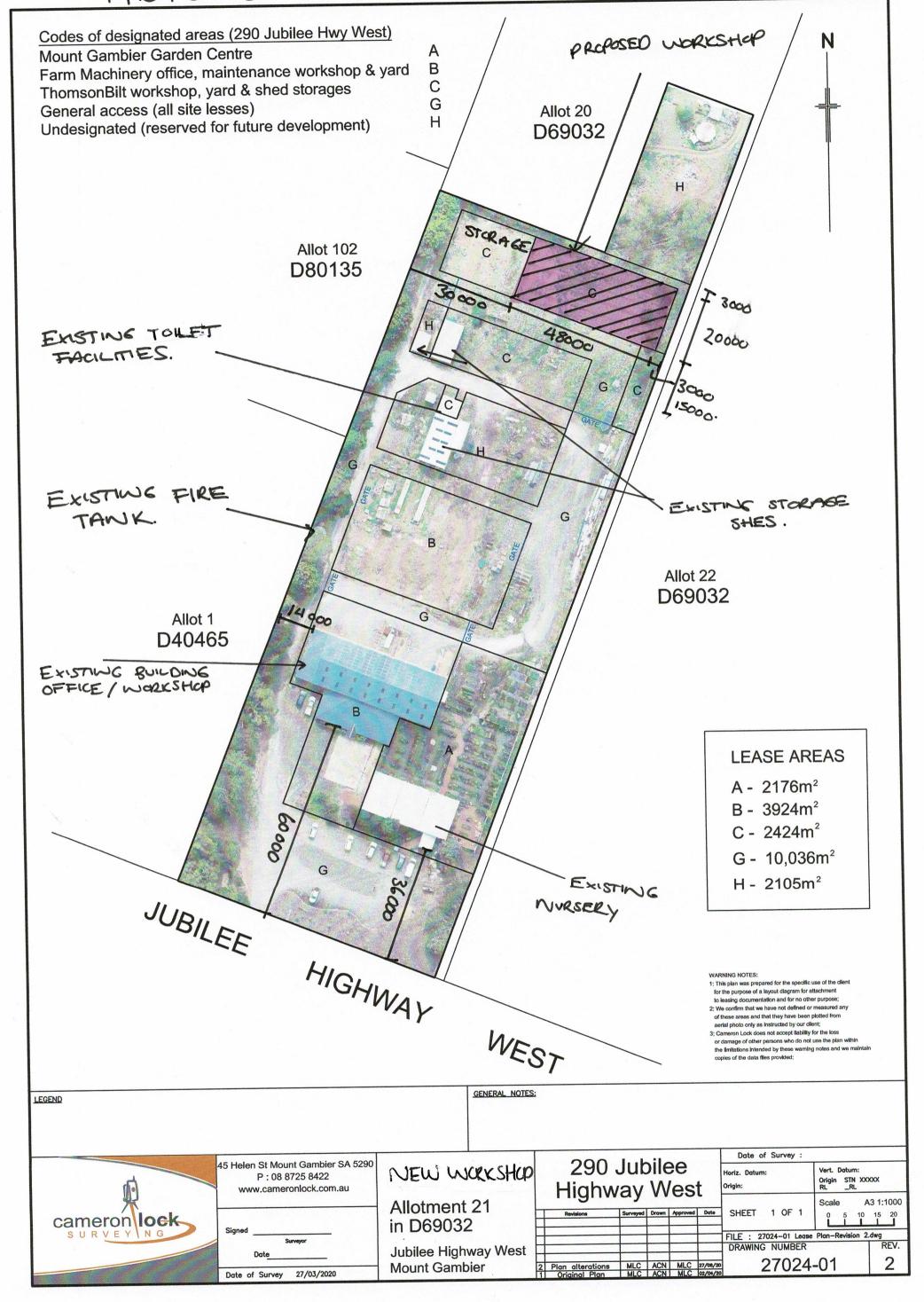
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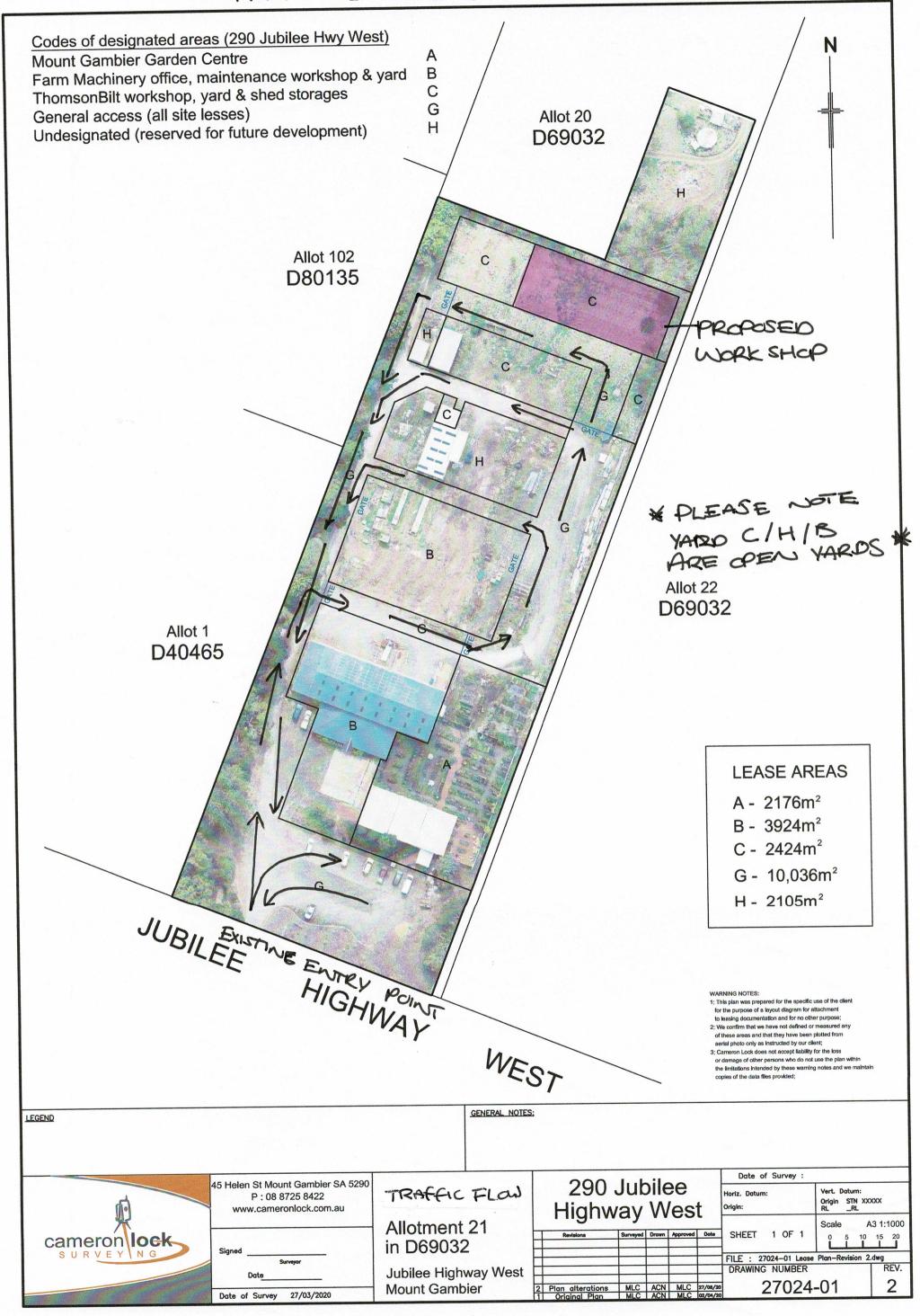
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MALPARA WORKSHOP 291 JUBILEE HIGHWAY WEST, MT. GAMBIER SA 5290 STRUCTURAL DRAWINGS

DRAWING LIST

COVER SHEET GENERAL NOTES

FOOTING PLAN

ROOF STEELWORK PLAN STEELWORK ELEVATIONS

CRANE LAYOUT PLAN

STEELWORK DETAILS - SHEET 1 STEELWORK DETAILS - SHEET 2 STEELWORK DETAILS - SHEET 3



ISSUED FOR APPROVAL

GENERAL NOTES

GENERAL

- G1 THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS' DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
- G2 ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE RELEVANT AND CURRENT STANDARDS AUSTRALIA CODES AND WITH THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITIES EXCEPT WHERE VARIED BY THE PROJECT SPECIFICATION.
- G3 ALL DIMENSIONS SHOWN SHALL BE VERIFIED BY THE BUILDER ON SITE. ENGINEER'S DRAWINGS SHALL NOT BE SCALED FOR DIMENSIONS. ENGINEER'S DRAWINGS ISSUED IN ANY ELECTRONIC FORMAT MUST NOT BE USED FOR DIMENSIONAL SETOUT. REFER TO THE ARCHITECT'S DRAWINGS FOR ALL DIMENSIONAL SETOUT INFORMATION.
- G4 DURING CONSTRUCTION THE STRUCTURE SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERSTRESSED. TEMPORARY BRACING SHALL BE PROVIDED BY THE BUILDER TO KEEP THE WORKS AND EXCAVATIONS STABLE AT ALL TIMES.
- G5 UNLESS NOTED OTHERWISE ALL LEVELS ARE IN METRES AND ALL DIMENSIONS ARE IN MILLIMETRES.
- G6 THE STRUCTURAL COMPONENTS DETAILED ON THESE DRAWINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RELEVANT STANDARDS AUSTRALIA CODES AND LOCAL GOVERNMENT ORDINANCES FOR THE LOADINGS NOTED IN THE STRUCTURAL DESIGN CRITERIA. REFER ARCHITECTURAL DRAWINGS FOR PROPOSED FLOOR USAGE.

FOUNDATIONS

- F1 FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE BEARING CAPACITY OF 150 kPa. THE FOUNDATION MATERIAL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER FOR THIS BEARING CAPACITY BEFORE PLACING MEMBRANE, REINFORCEMENT OR CONCRETE.
- F2 FOOTINGS SHALL BE LOCATED CENTRALLY UNDER WALL AND COLUMNS UNLESS NOTED OTHERWISE.
- F3 DO NOT EXCEED A RISE OF: 1 IN A RUN OF: 2 FOR THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS.
- F4 DO NOT BACKFILL RETAINING WALLS (OTHER THAN CANTILEVER WALLS) UNTIL FLOOR CONSTRUCTION AT TOP AND BOTTOM IS COMPLETED. ENSURE FREE DRAINING BACKFILL AND DRAINAGE IS IN PLACE
- F5 FOOTINGS TO BE CONSTRUCTED AND BACKFILLED AS OT INDITAVA YE SOUND LIDE FOLLOWING EXCAVATION TO AVOID SOFTENING OR DRYING OUT BY EXPOSURE.
- F6 FOOTINGS TO BE FOUNDED 200 MINIMUM INTO NATURAL GROUND LEVEL.

STRUCTURAL DESIGN CRITERIA

PERMANENT & IMPOSED LOADING (AS1170.1)

LC1 LOADING BASED ON USAGE

AREA	IMPOSED LOAD	SUPERIMPOSED PERMANENT LOAI
ROOF CRANE	0.25 kPa 5 T + 5 T	0.0 0.0

WIND DESIGN CRITERIA (AS1170.2)

C1	WIND REGION:	Α
	IMPORTANCE LEVEL (BCA):	2
	TERRAIN CATEGORY:	2
	SHIELDING CLASSIFICATION:	Ms = 1.0
	TOPOGRAPHIC CLASSIFICATION:	Mt = 1.0
	REGIONAL WIND SPEED:	Vu = 45 m/s
	CPI:	+0.1, -0.3

WC2 ROLLER SHUTTER AND OTHER DOORS SHALL BE DESIGNED AND CERTIFIED BY THE INSTALLER AND MANUFACTURER TO BE CAPABLE OF RESISTING A WIND PRESSURE OF:

AREA		REA PRESSURE (kPa)	
GENERAL PRESSURES		+1.01	-0.84
LOCAL PRESSURES	WA1	+1.37	
(REFER AS1170.2 CL5.4.4)	SA1	N/A	-1.21
(1.21.21.7.01.770.2 020.4.4)	SA2	N/A	-1.57

EARTHQUAKE DESIGN CRITERIA (AS1170.4)

EC1 THE RELEVANT PROVISIONS OF AS1170.4 HAVE BEEN APPLIED FOR EARTHQUAKE DESIGN

IMPORTANCE LEVEL:	2
FOUNDING MATERIAL:	Ce
HAZARD FACTOR:	0.10
DESIGN WORKING LIFE:	50 YEARS
EARTHQUAKE DESIGN CATEGORY:	II

CONCRETE

- C1 ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH AS3600 CURRENT EDITION WITH AMENDMENTS, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- C2 READYMIX CONCRETE SUPPLY SHALL COMPLY WITH AS 1379.
- C3 CONCRETE QUALITY, ALL THE REQUIREMENTS OF THE ACSE SPECIFICATION DOCUMENT 1 (EDITION 6), SHALL APPLY TO THE FORMWORK, REINFORCEMENT AND CONCRETE UNLESS NOTED OTHERWISE. CONCRETE QUALITY SPECIFICATIONS AS SHOWN ON PLAN.
- C4 PROJECT CONTROL TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH AS1379.
- C5 NO ADMIXTURES SHALL BE USED IN CONCRETE UNLESS APPROVED IN WRITING.
- C6 CLEAR CONCRETE COVER TO ALL REINFORCEMENT FOR DURABILITY SHALL BE AS FOLLOWS UNLESS SHOWN

OTHERWISE.						
		ENVIR	RONMENT			
GRADE	EXPOSED SURFACES		AGAINST GROUND			
	INTERIOR (A2)	EXTERIOR (B1)	WITH DPM (A1)	WITHOUT DPM (A2)		
25	30	60		30		
32	25	40	20	25		
40	20	30		20		

FOOTINGS COVERED WITH A SLAB ARE CONSIDERED INTERIOR EXPOSURE. FIRE RATING REQUIREMENTS MAY MEAN INCREASED COVER. REFER TO ENGINEER.

DURABILITY REQUIREMENTS FOR CONCRETE.

EXPOSURE	MINIMUM	MAXIMUM
COVER TO	CEMENT	W/C
AS3600:	CONTENT:	RATIO:
A1 & A2 B1	320	0.56 0.56
B2	390	0.46
C	450	0.40

C7 ALL REINFORCEMENT SHALL BE FIRMLY SUPPORTED ON MILD STEEL PLASTIC TIPPED CHAIRS, PLASTIC CHAIRS OR CONCRETE CHAIRS AT 1m MAX. CENTRES BOTH WAYS U.N.O. BARS SHALL BE TIED AT ALTERNATE INTERSECTIONS. USE PLASTIC CHAIRS IN EXPOSURE CONDITION GREATER THAN B1. MINIMUM BAR CHAIR SPACING FOR MESH REINFORCEMENT SHALL BE:

L92, SL102, SL81, RL918:	900 CTS
L72, SL82, RL818:	600 CTS

CONCRETE (CONTINUED)

- C8 CONCRETE SIZES DO NOT INCLUDE THICKNESSES OF APPLIED FINISHES.
- C9 DEPTHS OF BEAMS ARE GIVEN FIRST AND INCLUDE SLAB THICKNESS.
- C10 NO HOLES, CHASES OR EMBEDMENT OF PIPES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE PRIOR
- C11 CONSTRUCTION JOINTS WHERE NOT SHOWN SHALL BE LOCATED TO THE APPROVAL OF THE ENGINEER.

WRITTEN APPROVAL OF THE ENGINEER.

- C12 ALL CONCRETE SHALL BE COMPACTED WITH MECHANICAL VIBRATORS.
- C13 THE ENGINEER SHALL BE GIVEN 24 HOURS NOTICE FOR REINFORCEMENT INSPECTIONS AND CONCRETE SHALL NOT BE DELIVERED UNTIL ENGINEERS APPROVAL IS OBTAINED.
- C14 WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE ENGINEER.
- C15 REINFORCEMENT BARS AND LIGATURES: HOT ROLLED DEFORMED BAR, GRADE 500 NORMAL DUCTILITY AS4671-DN500N
 - HOT ROLLED ROUND BAR, GRADE 250 NORMAL DUCTILITY AS4671-R250N
 - COLD DRAWN ROUND WIRE, GRADE 500 LOW DUCTILITY AS467-R500L
 - POOL STEEL HOT ROLLED DEFORMED BAR, GRADE 250 NORMAL DUCTILITY AS4671-D250N

NOTE: THE UNDERSCORE REPRESENTS NOMINAL BAR DIAMETER IN ACCORDANCE WITH AS4671

- REINFORCEMENT FABRIC: SQUARE MESH, COLD DRAWN RIBBED WIRE GRADE 500, LOW DUCTILITY AS4671-D500L
- RECTANGULAR MESH, COLD DRAWN RIBBED WIRE GRADE 500, LOW DUCTILITY AS4671-D500L
- _L_TM TRENCH MESH, COLD DRAWN RIBBED WIRE GRADE 500, LOW DUCTILITY AS4671-D500L

NOTE: THE UNDERSCORE REPRESENTS VARYING SPECIFICATIONS IN ACCORDANCE WITH AS4671

- C16 REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY AND NOT NECESSARILY IN TRUE PROJECTION.
- C17 SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN POSITIONS SHOWN OR OTHERWISE APPROVED IN WRITING BY THE ENGINEER. LAPS SHALL BE IN ACCORDANCE WITH AS3600 AND NOT LESS THAN THE DEVELOPMENT LENGTH FOR EACH BAR.
- C18 STANDARD LAP AND COG LENGTHS UNLESS NOTED OTHERWISE ON DRAWINGS:

BAR DIAMETER	MIN. LAP LENGTH (mm)	MIN. COG LENGTH (mm)
N12	500	180
N16	750	210
N20	1000	260
N24	1375	310
N28	1560	360
N32	1810	400

STRUCTURAL STEEL

- S1 ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH AS4100 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- S2 UNLESS NOTED OTHERWISE, ALL STEEL SHALL BE IN ACCORDANCE WITH AS3678 GRADE 250, OR AS3679 GRADE 300, OR AS1163 GRADE 350 AS APPROPRIATE.
- S3 STRUCTURAL STEELWORK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AUSTRALIAN STANDARDS AS/NZS5131 FOR A CONSTRUCTION CATEGORY OF CC2.
- S4 ELECTRONIC PDF WORKSHOP FABRICATION DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AT LEAST 7 DAYS PRIOR TO COMMENCEMENT OF FABRICATION. FABRICATION IS NOT TO COMMENCE WITHOUT ENGINEER'S APPROVAL OF WORKSHOP DRAWINGS. WHERE NOT INDICATED ON STRUCTURAL DRAWINGS, ALL DIMENSIONS & SETOUT TO BE OBTAINED FROM ARCHITECTURAL DRAWINGS.
- S5 BOLTS ARE DESIGNATED ON THE DRAWINGS BY THE NUMBER, DIAMETER, GRADE AND TIGHTENING PROCEDURE. 4.6/S DENOTES COMMERCIAL BOLTS OF GRADE 4.6 TO AS1111, SNUG TIGHTENED. 8.8/S DENOTES HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO AS1252, SNUG TIGHTENED. 8.8/TB DENOTES HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO AS1252, FULLY TENSIONED TO AS4100 AS A BEARING TYPE JOINT 8.8/TF DENOTES HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO AS1252 FULLY TENSIONED TO AS4100 AS A FRICTION TYPE JOINT WITH FACING SURFACES LEFT UNCOATED.
- S6 UNLESS NOTED OTHERWISE ALL BOLTS SHALL BE M20 CATEGORY 8.8/S. NO CONNECTION SHALL HAVE LESS THAN 2 BOLTS. ALL BOLTS AND WASHERS SHALL BE GALVANISED. CLEATS AND GUSSETS SHALL BE 10mm THICK.
- S7 FULLY TENSIONED BOLTS TO BE INSTALLED IN ACCORDANCE WITH SECTION 15 OF AS4100, USING THE PART-TURN OR THE DIRECT-TENSION INDICATOR METHOD.
- S8 FILLET WELDS SHALL BE 6mm CONTINUOUS, CATEGORY SP, USING ELECTRODES IN ACCORDANCE WITH AS1554.1 U.N.O. BUTT WELDS SHALL BE COMPLETE PENETRATION BUTT WELDS IN ACCORDANCE WITH AS1554.1 ALL OTHER WELDS SHALL BE IN ACCORDANCE WITH AS1554.1. WELD CATEGORY: PURLIN AND GIRT CLEATS: GP (E48XX) ALL OTHER U.N.O.:
- S9 ALL WELDS SHALL BE INSPECTED IN ACCORDANCE WITH AS1554.1. THE EXTENT OF NON DESTRUCTIVE EXAMINATION SHALL COMPLY WITH AS1554.1. DEFECTIVE WELDS SHALL BE REPAIRED OR REPLACED IN ACCORDANCE WITH AS1554.1
- \$10 PROVIDE SEAL PLATES TO THE ENDS OF ALL HOLLOW SECTIONS, WITH "BREATHER" HOLES IF MEMBERS TO BE HOT DIP GALVANISED.
- S11 ALL STEELWORK SHALL BE TEMPORARILY BRACED BY THE ERECTOR AS NECESSARY TO STABILISE THE STRUCTURE DURING ERECTION AND UNTIL PERMANENT STABILISING ELEMENTS HAVE BEEN CONSTRUCTED.
- S12 STRUCTURAL STEELWORK BELOW GROUND NOT ENCASED IN CONCRETE: AS2312.1 SYSTEM: EVH3 ABRASIVE BLAST CLEAN TO AS1627.4 CLASS 2.5 TWO COATS 250 MICRON DULUX DUREBILD HSE OR EQUIV.
- S13 STRUCTURAL STEELWORK BELOW GROUND ENCASED IN CONCRETE SHALL BE UNPAINTED. 75mm MINIMUM CONCRETE THICKNESS FROM STEEL, 25MPa CONCRETE.
- \$14 COLD ROLLED PURLINS/GIRTS ARE TO BE INSTALLED COMPLETE WITH BRIDGING ETC IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
- \$15 STRUCTURAL STEELWORK NOT ENCASED IN CONCRETE SHALL HAVE THE FOLLOWING SURFACE TREATMENT IN ACCORDANCE WITH THE SPECIFICATION:

INTERNAL - AS2312.1-2014 SYSTEM: IZS1/2 ABRASIVE BLAST CLEAN AS 1627.4 CLASS 2.5 1 COAT 75 MICRON INORGANIC ZINC SILICATE COMPATIBLE TOP COAT IF REQUIRED.

EXTERNAL (WAREHOUSE) - AS2312.2-2014: HDG600 CLEAN STEELWORK BY REMOVING ALL CARBONACEOUS FILMS (OIL, GREASE, PAINT, WELDING SLAG, ETC.) PRIOR TO PICKLING. HOT DIP GALVANISE TO MIN. 85 MICRON IN ACCORDANCE WITH AS4680-2006.

PURLINS & GIRTS - AS2312.2-2014 SYSTEM: Z350 HOT DIP ZINC COATED SHEET IN ACCORDANCE WITH AS1397-2011.

STRUCTURAL STEEL (CONTINUED)

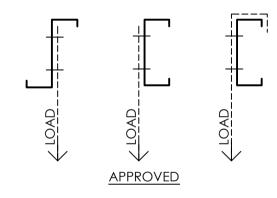
- \$16 ALL GALVANISING OF STRUCTURAL STEELWORK SHALL BE PROCESSED IN ACCORDANCE WITH AS4680/2006 'GALVANIZED COATINGS ON FABRICATED FERROUS ARTICLES'. THE CONTINUOUS AVERAGE ZINC COATING MASS TO BE 600G/M² (550G/M² MINIMUM).
- S17 THE PURLIN/GIRT SYSTEM IS TO BE COMPRISED OF LYSAGHT CEE AND ZED SECTIONS (WITH STANDARD Z350 COATING) INCLUDING HOOK-LOK II BRIDGING SYSTEM AND GRADE 4.6 M12 BOLTS WITH INTEGRAL WASHERS FOR BOTH HEAD AND NUT. IF OTHER PURLIN/GIRT SYSTEMS ARE PROPOSED THEN WRITTEN PERMISSION MUST BE SOUGHT FROM THE ENGINEER.
- \$18 THE BUILDER IS TO ENSURE THAT THE ERECTION OF THE STRUCTURAL STEELWORK IS IN STRICT ACCORDANCE WITH AS3828. 1998 'GUIDELINES FOR THE ERECTION OF BUILDING STEELWORK'. ALLOW FOR THE INSTALLMENT OF TEMPORARY BRACING AS REQUIRED.

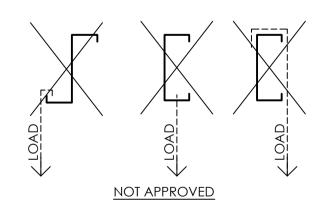
COLD-FORMED STRUCTURAL STEEL

- CF1 ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH AS4600 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS
- CF2 UNLESS NOTED OTHERWISE, ALL STEEL SHALL BE IN ACCORDANCE WITH AS1163, AS1397 (EXCLUDING GRADE G550, LESS THAN 0.9mm IN THICKNESS), AS/NZS 1594, AS/NZS 1595 AND AS/NZS 3678, AS APPROPRIATE.
- CF3 BOLTS ARE DESIGNATED ON THE DRAWINGS BY THE NUMBER, DIAMETER, GRADE AND TIGHTENING PROCEDURE. STEEL BOLTS, NUTS AND WASHERS SHALL COMPLY WITH AS1110.1, AS1111.1, AS1112.1, AS1112.2, AS1112.3, AS1112.4, AS/NZS 1252, AS/NZS 1559 AND AS 4291.1 (ISO 898-1), AS APPROPRIATE.
- CF4 UNLESS NOTED OTHER WISE, ALL BOLTS SHALL BE M12 CATEGORY 4.6/S. UNLESS NOTED OTHERWISE, NO CONNECTION SHALL HAVE LESS THAN 2 BOLTS. ALL BOLTS AND WASHERS SHALL BE GALVANISED. UNLESS NOTED OTHERWISE, CLEATS AND GUSSETS SHALL BE 6mm THICK.
- CF5 FILLET WELDS SHALL BE 3mm CONTINUOUS USING ELECTRODES AS1554.1 U.N.O BUTT WELDS SHALL BE COMPLETE PENETRATION BUTT WELDS IN ACCORDANCE WITH AS1554.1. ALL WELDS AND WELD TESTING, INCLUDING EXTEND OF NON-DESTRUCTIVE EXAMINATION AND REPAIR OF DEFECTIVE WELDS SHALL BE IN ACCORDANCE WITH AS1554.1.

PURLIN LOADING

PL1 THE SUSPENSION OF CEILINGS, SERVICES ETC FROM PURLINS SHALL BE IN ACCORDANCE WITH THE APPROVED METHODS DETAILED BELOW;





NOTE:

THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL & OTHER CONSULTANTS' DRAWINGS & SPECIFICATIONS & WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

NOTE:

NOTE:

ALL EXPOSED STEELWORK

TO BE HOT DIP GALVANISED

ALL BOLTS TO BE GRADE 8.8/S U.N.O. ALL BOLTED CONNECTIONS TO BE 10 FIN PLATE + 2M20 BOLTS U.N.O.

MEGASPAN SHEDS 290 JUBILEE HWY WEST.

MALPARA WORKSHOP 291 JUBILEE HIGHWAY WEST MT. GAMBIER SA 5290

DRAWN



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71 GLEN OSMOND ROAD, EASTWOOD SA 5063 PO BOX 474, TANUNDA SA 5352

SYDNEY | ADELAIDE | DARWIN | PARRAMATTA | MUDGEE | BAROSSA

GENERAL NOTES

PROJECT No. DRAWING No.

DATE ISSUE BY

ISSUED FOR APPROVAL

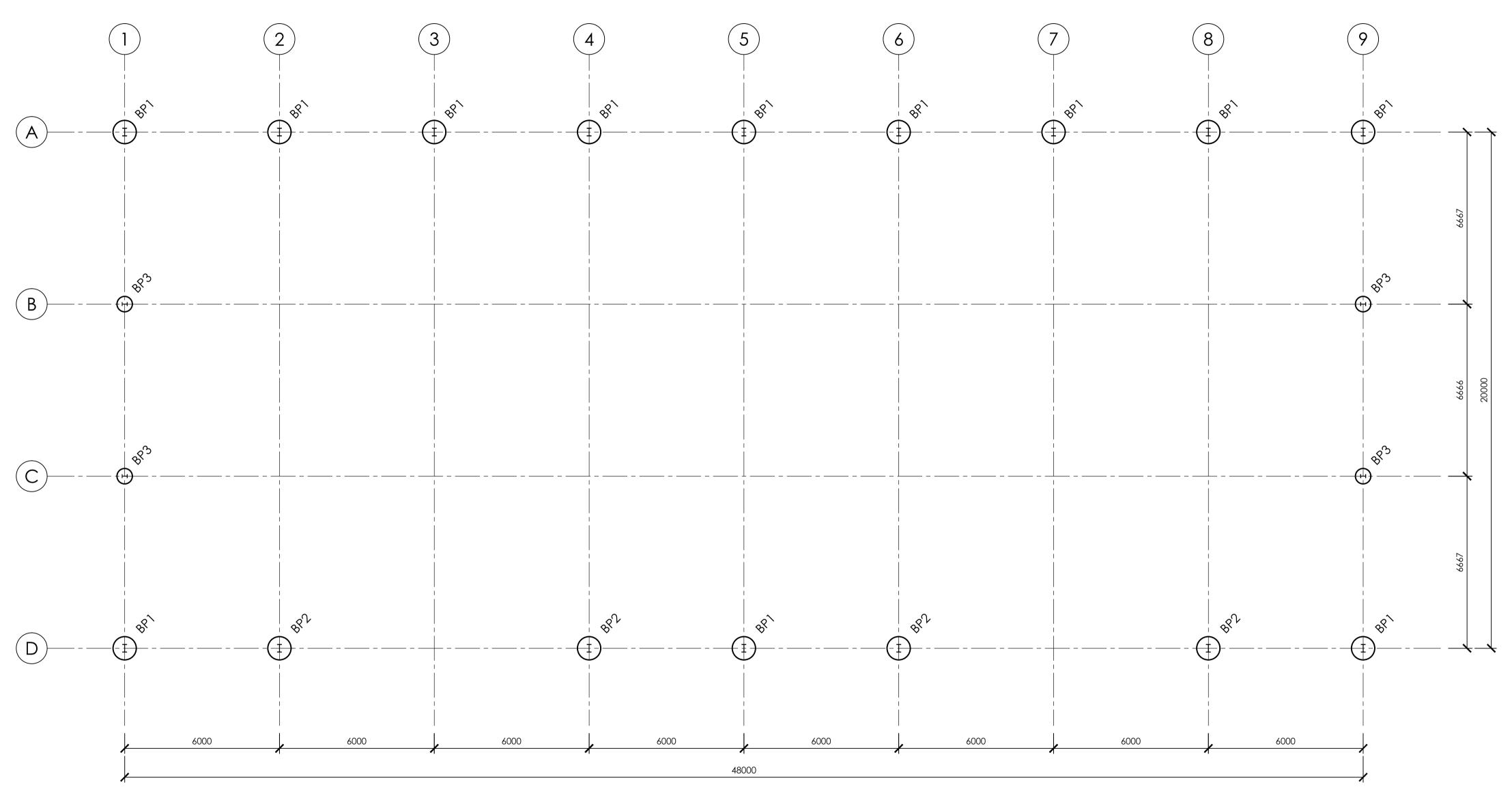
AMENDMENTS

DESIGNED

FI.T.

FOOTING SCHEDULE					
MARK	TYPE	size (minimum)	REINFORCEMENT		
BP1	BORED PIER	Ø900 × 1600 DEEP	N.A. MASS CONCRETE		
BP2	BORED PIER	Ø900 × 1800 DEEP	N.A. MASS CONCRETE		
BP3	BORED PIER	Ø600 × 900 DEEP	n.a. mass concrete		

FOOTING CONCRETE QUALITY								
ELEMENT	AS3600 EXPOSURE CLASS.	AS1379 CONCRETE CLASS.	f'c (MPa) CHARACTERISTIC COMPRESSIVE STRENGTH	CEMENT TYPE	ADMIXTURES	MAX. AGGREGATE SIZE (mm)	SLUMP	REMARKS
BORED PIERS	A2	Ν	25	GP	NIL	20	80	



FOOTING PLAN SCALE 1:100 AT A1

FOOTING NOTES:

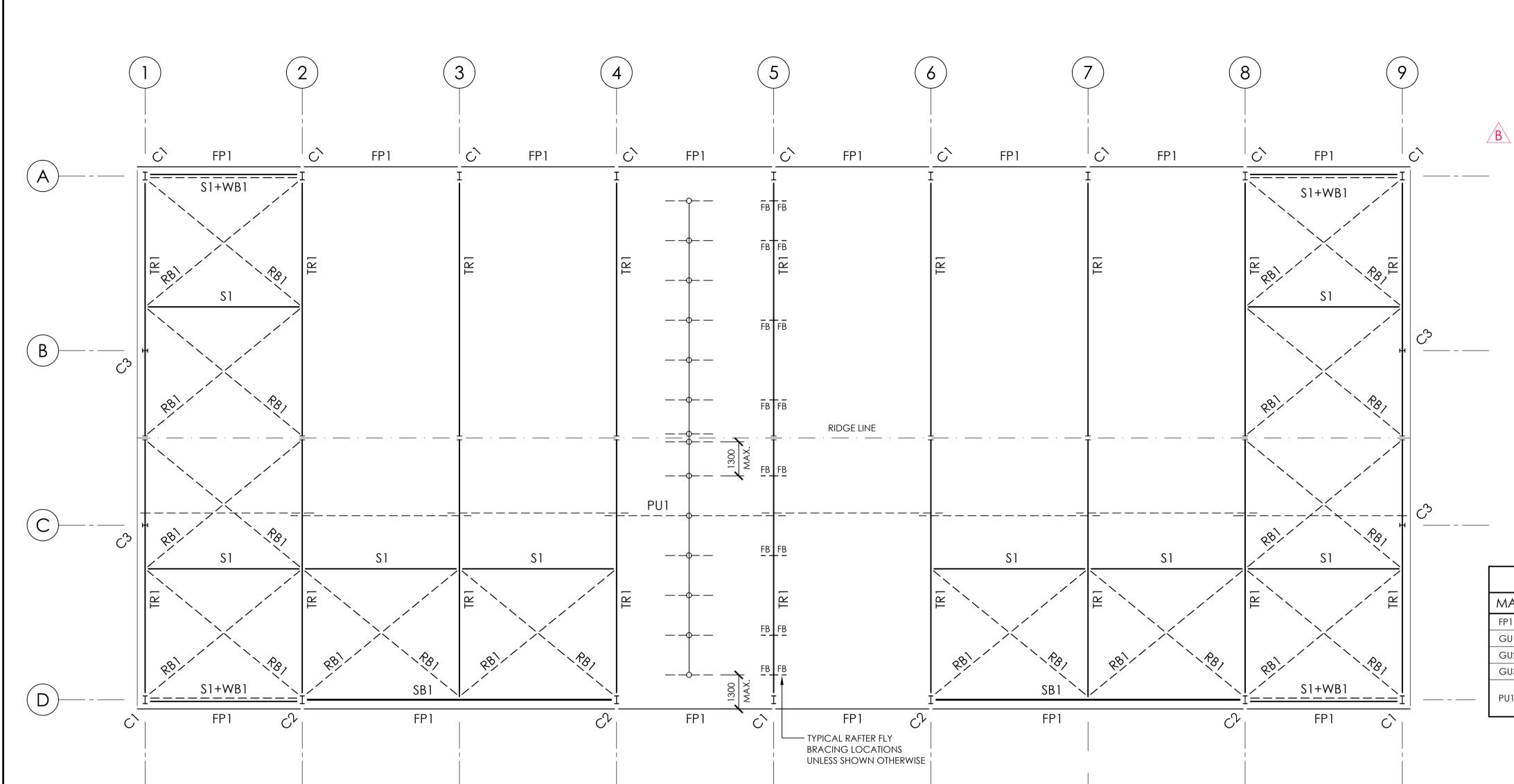
- TOP OF FOOTING LEVEL TO BE ADVISED BY CLIENT.
- LOCALLY DEEPEN FOOTINGS TO ACHIEVE MINIMUM 50 CONCRETE COVER TO ALL ANCHOR BOLTS & CAST-IN ITEMS
- FOOTINGS TO BE FOUNDED 200 MIN. INTO NATURAL GROUND WITH MINIMUM 150 KPa ALLOWABLE BEARING CAPACITY. DEEPEN FOOTINGS AS REQUIRED OR REFER TO ENGINEER.
- ALTERNATIVE: FOOTINGS TO BE FOUNDED 200 MINIMUM INTO LEVEL 1 CONTROLLED FILL WITH MINIMUM 100kPa ALLOWABLE BEARING CAPACITY. ALL STRUCTURAL STEELWORK TO BE CONCRETE ENCASED OR HAVE APPROPRIATE PROTECTION COATING IN ACCORDANCE WITH AS2312.1/.2, PRIOR TO BACKFILLING.
- REFER TO STRUCTURAL STEEL NOTES.
- ALL FOOTING REINFORCEMENT TO HAVE 50 MINIMUM CLEAR CONCRETE COVER

0 1 2 3 4 5 6 7 8 9 10m

SCALE 1:100 AT A1 SHEET | 1:200 AT A3 SHEET

FOOTING PLAN

ISSUED FOR APPROVAL



	STEELWORK MEMBER SCHEDULE								
MARK	MEMBER	SIZE	REMARKS						
C1	COLUMN	360 UB 45							
C2	COLUMN	360 UB 51							
C3	COLUMN	200 UB 22							
CB1	CRANE BEAM	700 WB 173 + 2/380 PFC I	12m SPAN CRANE BEAM						
CB2	CRANE BEAM	360 UB 57 + 300 PFC	6m SPAN CRANE BEAM						
CS1	CRANE BEAM SUPPORT	360 UB 45							
DH1	DOOR HEADER	300 PFC							
FB	FLY BRACING	50×50×3.0 EA	REFER TO DETAIL						
RB1	ROOF BRACING	M16 4.6/S THREADED ROD							
\$1	STRUT	75×75×3.5 SHS							
TM1	TRIMMER	C150-15							
WB1	WALL BRACING	M16 4.6/S THREADED ROD							

TRU	TRUSS MEMBER SCHEDULE TR1			
MARK	MEMBER	SIZE		
BC1	BTM. CHORD	89×89×5.0 SHS		
TC1	TOP CHORD	89×89×5.0 SHS		
WE1	WEB	50×50×3.0 SHS		
NOTE: TR1 DEP	NOTE: TR1 DEPTH = 600mm			

SPAN BEAM MEMBER SCHEDULE SB1						
MARK MEMBER SIZE						
BC2	BTM. CHORD	125×125×6.0 SHS				
TC2	TOP CHORD	125×125×5.0 SHS				
WE2 WEB 50×50×3.0 SHS						
NOTE: SB1 DEPTH = 600mm						

PURLIN/GIRT SCHEDULE						
MARK	MEMBER	SIZE	CENTRES	LAP	BRIDGING	
FP1	FASCIA PURLIN	C150-19	-	-	1 ROW	
GU1	WALL GIRTS	Z150-15	1700 MAX.	900 MIN.	1 ROW + GIRT FEET	
GU2	WALL GIRTS	Z150-19	1700 MAX.	900 MIN.	1 ROW + GIRT FEET	
GU3	WALL GIRTS	Z150-24	1700 MAX.	SINGLE SPAN	2 ROWS + GIRT FEET	
PU1	ROOF PURLINS	Z150-15	FIRST PURLIN BAY AT EAVES & RIDGE TO BE 1300 MAX. INTERNAL BAYS AT 1600 MAX. CTS.	900 MIN.	1 ROW	

CLADDING SPECIFICATION

TRIMDEK 0.42 BMT

WALL: TRIMDEK 0.42 BMT

INSTALLED AS PER MANUFACTURER'S SPECIFICATIONS

NOTE:

CONFIRM ALL DIMENSIONS WITH ARCHITECT DRAWINGS. ARCHITECT DRAWINGS TO TAKE PRECEDENCE.

ROOF STEELWORK PLAN

SCALE 1:100 AT A1

PURLIN/GIRT SETOUT NOTE:

THE PURLIN/GIRT LAYOUT SHOWN IS DIAGRAMMATIC ONLY.

THE PURLIN/GIRT CENTRES NOMINATED ARE MAXIMUM AND MAY NEED TO BE REDUCED TO COMPLY WITH SUPPORT SPACING REQUIRED BY ROOF SHEET MANUFACTURER AND/OR FOR ARCHITECTURAL FINISHES ETC. REFER TO ARCHITECTURAL DOCUMENTATION FOR TYPE AND THICKNESS OF ROOF SHEETING AND FINISHES. THE FABRICATOR SHALL DETERMINE THE TOTAL NUMBER OF PURLIN/GIRT RUNS REQUIRED.

THE TOTAL NUMBER OF PURLIN/GIRT RUNS REQUIRED SHALL BE BASED ON THE WRITTEN INFORMATION

CONTAINED ON STRUCTURAL AND ARCHITECTURAL DOCUMENTATION.

THE FABRICATOR SHALL PROVIDE ALL TRIMMING ANGLES REQUIRED FOR THE SUPPORT OF BOX GUTTERS, FLASHING ETC. AND THE EDGE AND END SUPPORT OF ROOF AND WALL SHEETING.

PORTAL DESIGN CRITERIA NOTE:

THE PORTAL FRAME HAS BEEN DESIGNED FOR THE FOLLOWING LOADING CONDITIONS IN ADDITION TO THE NOTED WIND AND EARTHQUAKE DESIGN CRITERIA:

TYPE	LOAD (kPa)		
PERMANENT LOAD	0.10 (GENERAL AREAS)		
IMPOSED LOADS	0.25 (NON-TRAFFICABLE)		

ROD BRACING NOTE:

ALL ROD BRACING TO BE CLIPPED TO UNDERSIDE OF ROOF PURLINS

0 1 2 3 4 5 6 7 8 9 10m

SCALE 1:100 AT A1 SHEET | 1:200 AT A3 SHEET

ISSUED FOR APPROVAL ISSUED FOR APPROVAL **AMENDMENTS**

DATE ISSUE BY

MEGASPAN SHEDS 290 JUBILEE HWY WEST, MT. GAMBIER SA 5290

MALPARA PTY LTD

MALPARA WORKSHOP 291 JUBILEE HIGHWAY WEST

MT. GAMBIER SA 5290 DESIGNED DRAWN DATE S.S. 31.08.20 A1 TX12805.49 - S01

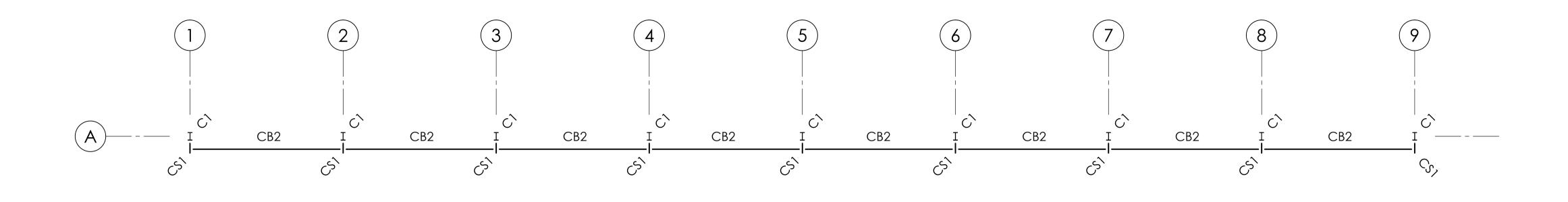


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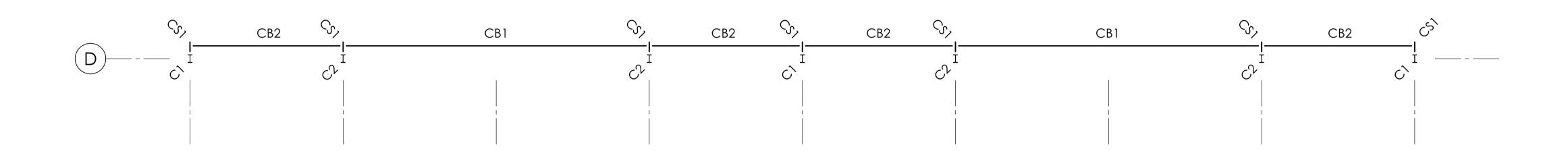
ROOF STEELWORK PLAN

PROJECT No. TX12805.49 - S3.01 B SYDNEY | ADELAIDE | DARWIN | PARRAMATTA | MUDGEE | BAROSSA





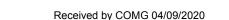


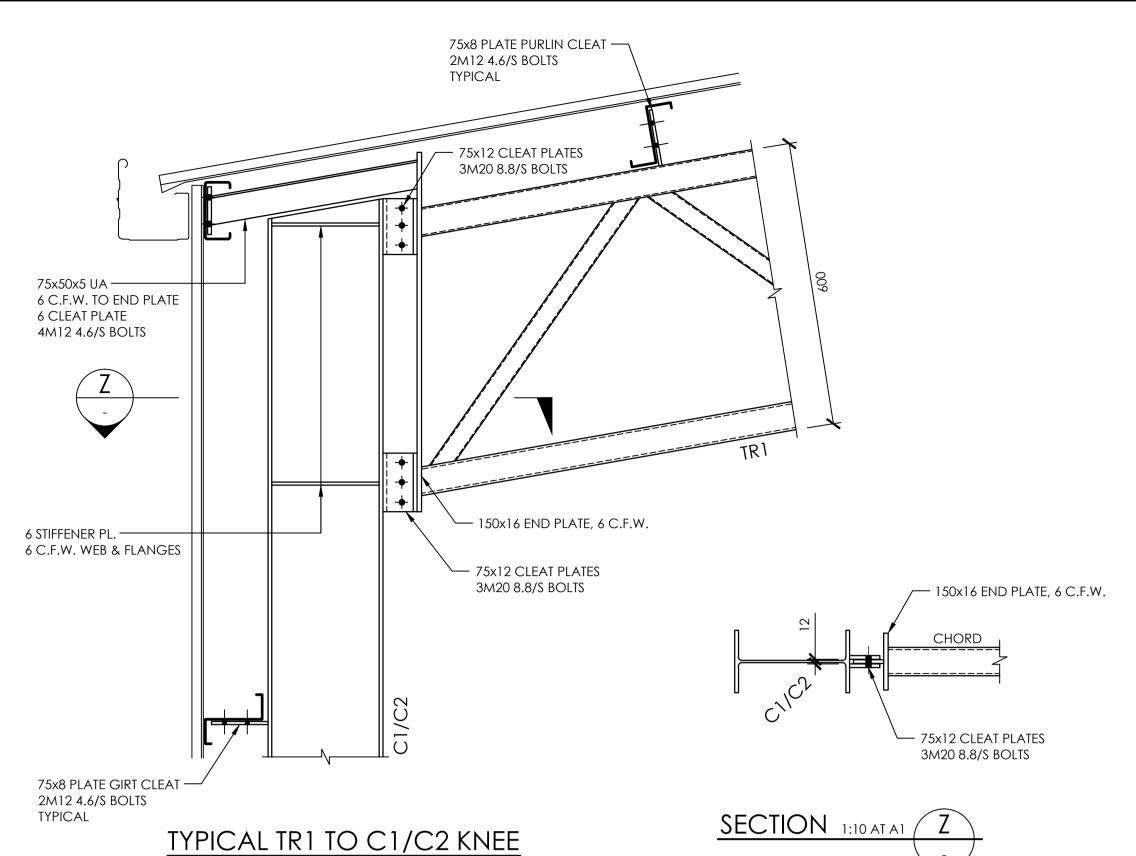


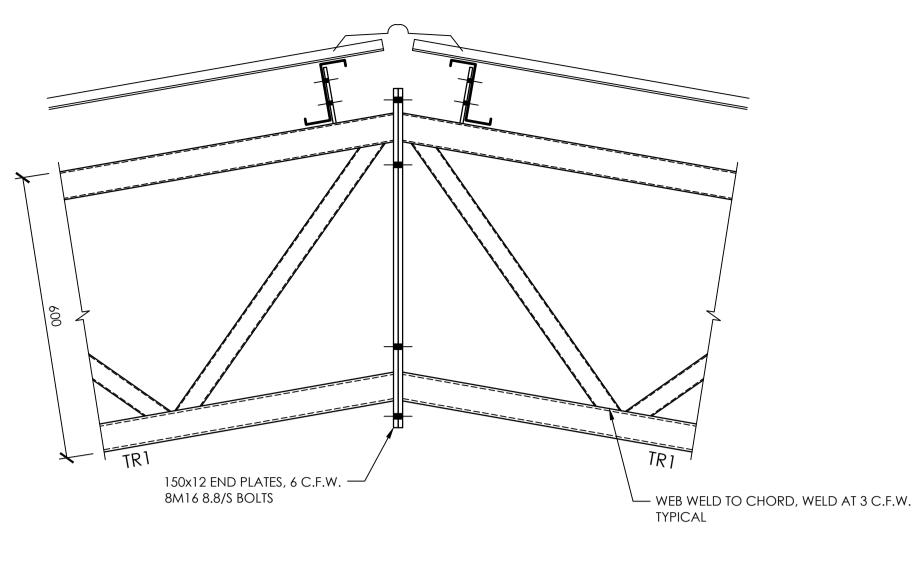
CRANE LAYOUT PLAN SCALE 1:100 AT A1

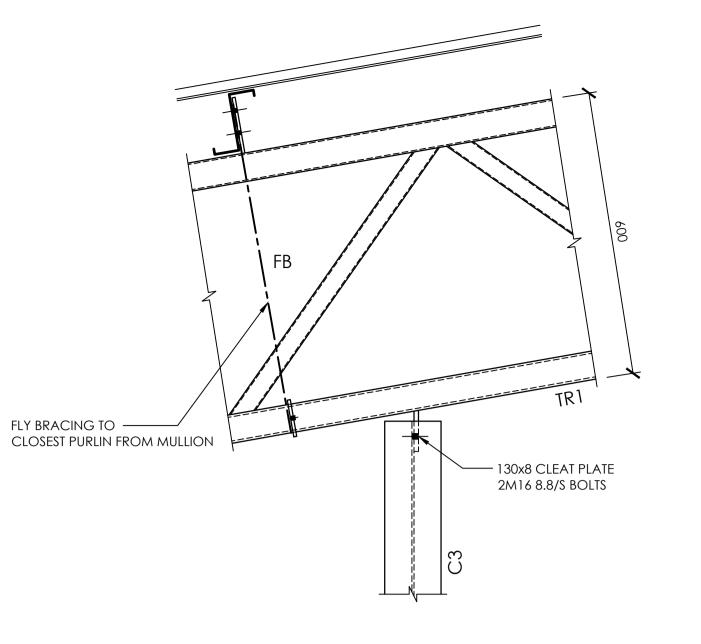
0 1 2 3 4 5 6 7 8 9 10m

SCALE 1:100 AT A1 SHEET | 1:200 AT A3 SHEET







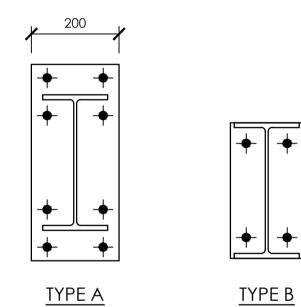


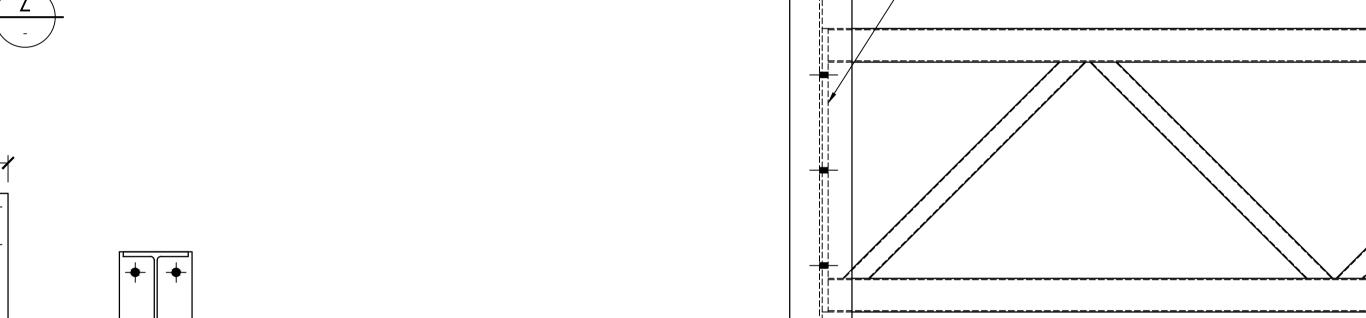
TYPICAL TR1 TO TR1 RIDGE CONNECTION DETAIL SCALE 1:10 AT A1

TYPICAL END WALL COLUMN CONNECTION DETAIL SCALE 1:10 AT A1

SB1

CONNECTION DETAIL SCALE 1:10 AT A1





COLUMN BASE PLATE DETAIL

ON FOOTINGS SCALE 1:10 AT A1

NOTE:

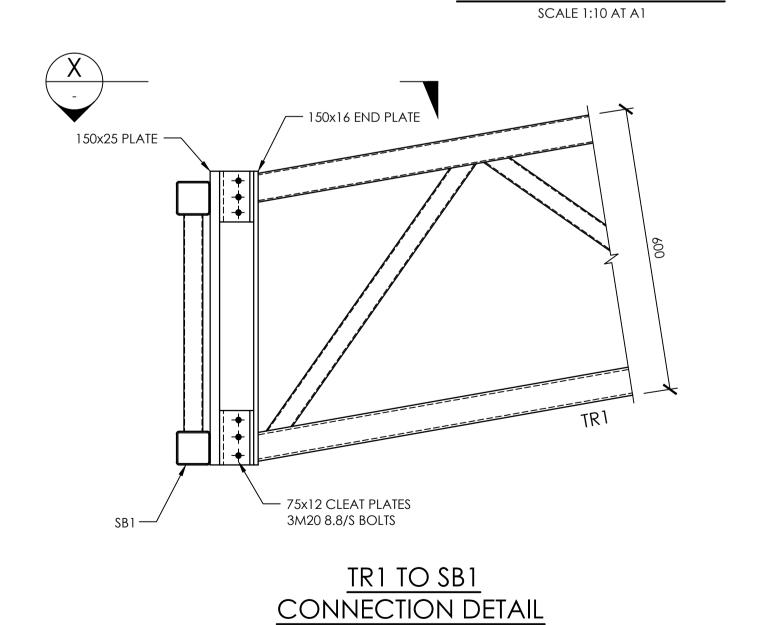
6 C.F.W. CATEGORY SP TO COLUMN FLANGES (U.N.O.) 6 C.F.W. CATEGORY SP TO COLUMN TO WEB (U.N.O.) ALL BASE PLATE GROUT TO BE HIGH-STRENGTH, NON-SHRINK GROUT (>50 MPa)

20 mm NON-SHRINK GROUT	COLUMN
50x6 FLAT EACH FACE 16 DIAMETER ROD EXTENDING 50 mm PAST THE BOLT CENTER	REFER TO DETAIL FOR EMBEDMENT
16 DIAMETER ROD - BOTH SIDES	

6 DIAMETER ROD ——————————————————————————————————	COLUMN BASE PLATE SCHEDULE						
			BASE PLAT	Ē	AN	CHORS	
TYPICAL BASE PLATE DETAIL	MEMBER	TYPE	THICKNESS	GROUT	ANCHOR BOLTS	TYPE (REFER SPEC.)	REMARKS
<u>ELEVATION</u>	C1	Α	20mm	20mm	8M20 4.6/S	CAST-IN	
SCALE 1:10 AT A1	C2	Α	20mm	20mm	8M20 4.6/S	CAST-IN	
	C3	В	16mm	20mm	4M16 4.6/S	CAST-IN	

MEMBER	BASE PLATE			ANCHORS		
	TYPE	THICKNESS	GROUT	ANCHOR BOLTS	TYPE (REFER SPEC.)	REMARKS
C1	Α	20mm	20mm	8M20 4.6/S	CAST-IN	
C2	Α	20mm	20mm	8M20 4.6/S	CAST-IN	
C3	В	16mm	20mm	4M16 4.6/S	CAST-IN	

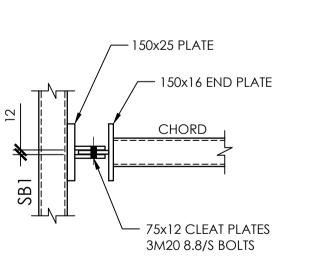
ANCHOR SPECIFICATIONS							
TYPE	ANCHOR	REMARKS					
CAST-IN	M16 4.6/S	550	N/A				
BOLTS	M20 4.6/S	690	N/A				



150x16 END PLATE, 6 C.F.W.
 6M16 8.8/S BOLTS

SB1 TO C2

CONNECTION DETAIL



SECTION 1:10 AT A1 X

MALPARA PTY LTD

MALPARA WORKSHOP 291 JUBILEE HIGHWAY WEST MT. GAMBIER SA 5290

SIZE A1 DRAWN DATE S.S. 31.08.20 TX12805.49 - S01

TRIAXIAL CONSULTING COMPLEX PROBLEMS RESOLVED SIMPLY

SCALE 1:10 AT A1

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SHEET 1 71 GLEN OSMOND ROAD, EASTWOOD SA 5063 PO BOX 474, TANUNDA SA 5352

STEELWORK DETAILS

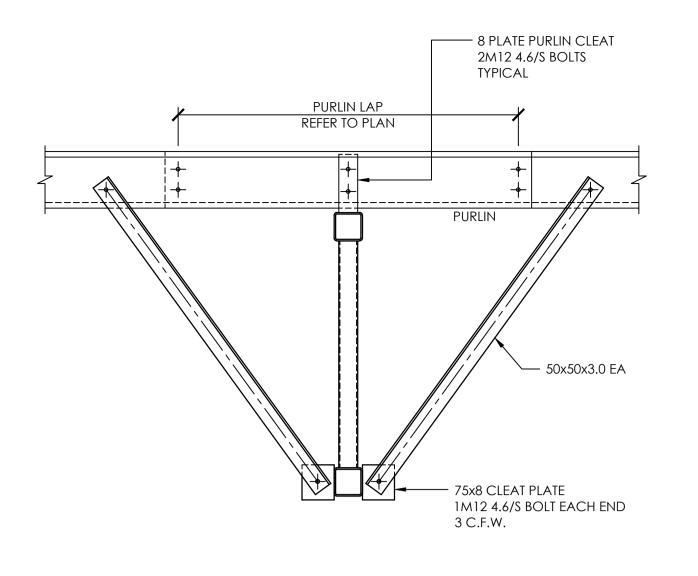
SCALE 1:10 AT A1 SHEET | 1:20 AT A3 SHEET

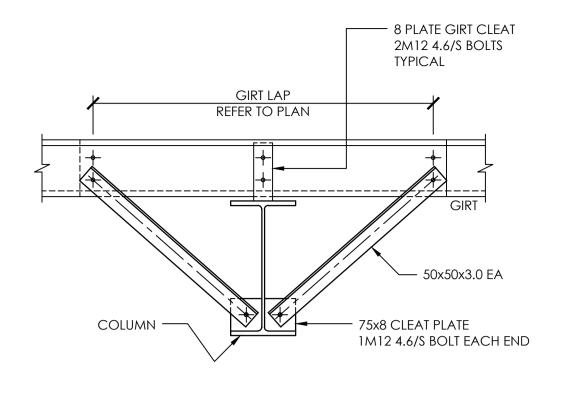
PROJECT No. SYDNEY | ADELAIDE | DARWIN | PARRAMATTA | MUDGEE | BAROSSA

MEGASPAN SHEDS 290 JUBILEE HWY WEST, MT. GAMBIER SA 5290 DATE ISSUE BY

FOR CONSTRUCTION SUBJECT TO COUNCIL APPROVAL

ISSUED FOR APPROVAL

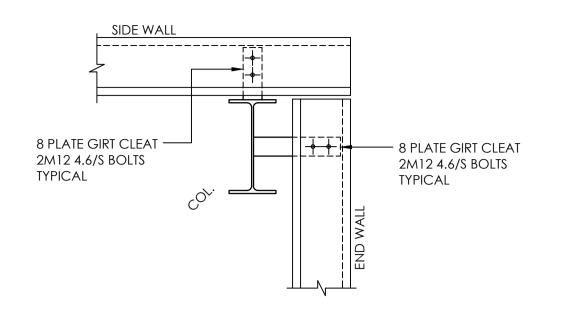


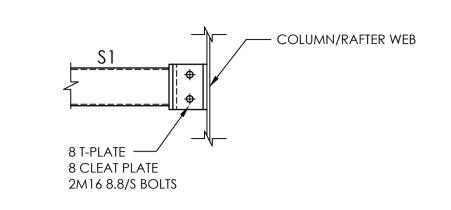


TYPICAL FLY BRACING (FB) DETAIL (FOR COLUMN)

SCALE 1:10 AT A1

DENOTED FB ON ELEVATIONS



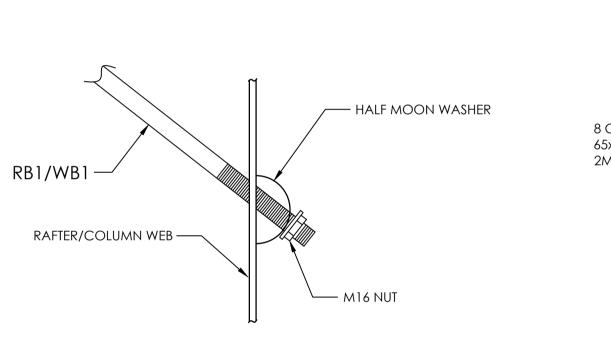


TYPICAL COLUMN CORNER DETAIL SCALE 1:10 AT A1

TYPICAL STRUT END CONNECTION DETAIL SCALE 1:10 AT A1

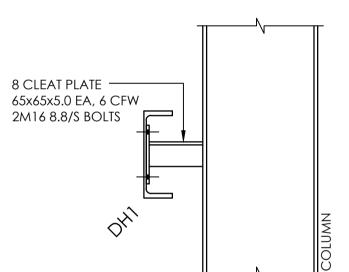
TYPICAL FLY BRACING (FB) DETAIL (FOR TRUSS)

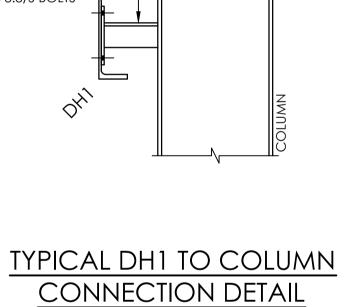
SCALE 1:10 AT A1 DENOTED FB ON PLAN



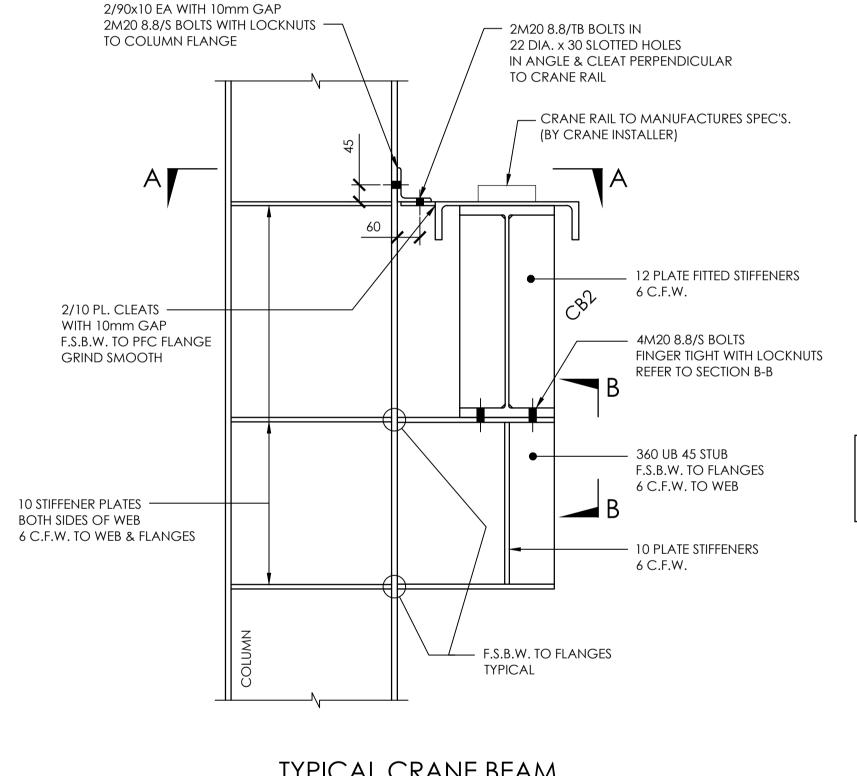
TYPICAL BRACING END

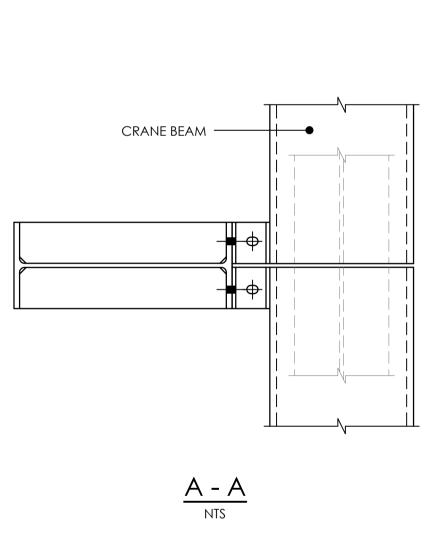
CONNECTION DETAIL

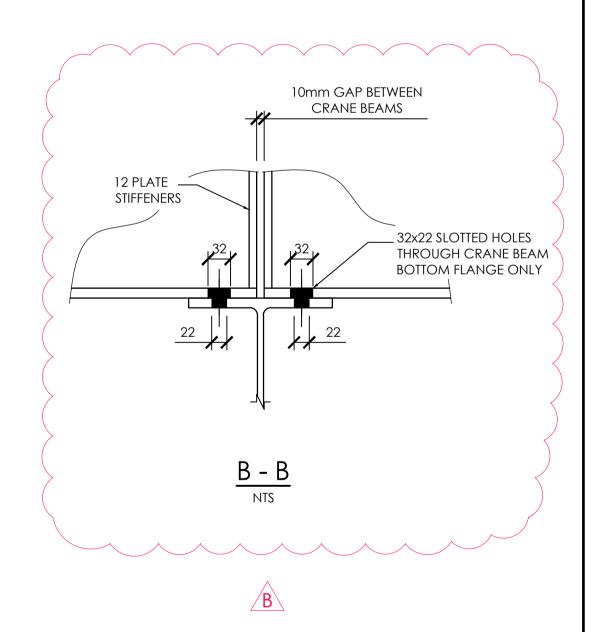




SCALE 1:10 AT A1







TYPICAL CRANE BEAM CONNECTION DETAIL

SCALE 1:10 AT A1 SHEET | 1:20 AT A3 SHEET

MEGASPAN SHEDS 290 JUBILEE HWY WEST,

MT. GAMBIER SA 5290

MALPARA PTY LTD

MALPARA WORKSHOP 291 JUBILEE HIGHWAY WEST MT. GAMBIER SA 5290

A1 TX12805.49 - S01

TRIAXIAL CONSULTING PO BOX 474, TANUNDA SA 5352 COMPLEX PROBLEMS RESOLVED SIMPLY SYDNEY | ADELAIDE | DARWIN | PARRAMATTA | MUDGEE | BAROSSA

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STEELWORK DETAILS SHEET 2 71 GLEN OSMOND ROAD, EASTWOOD SA 5063

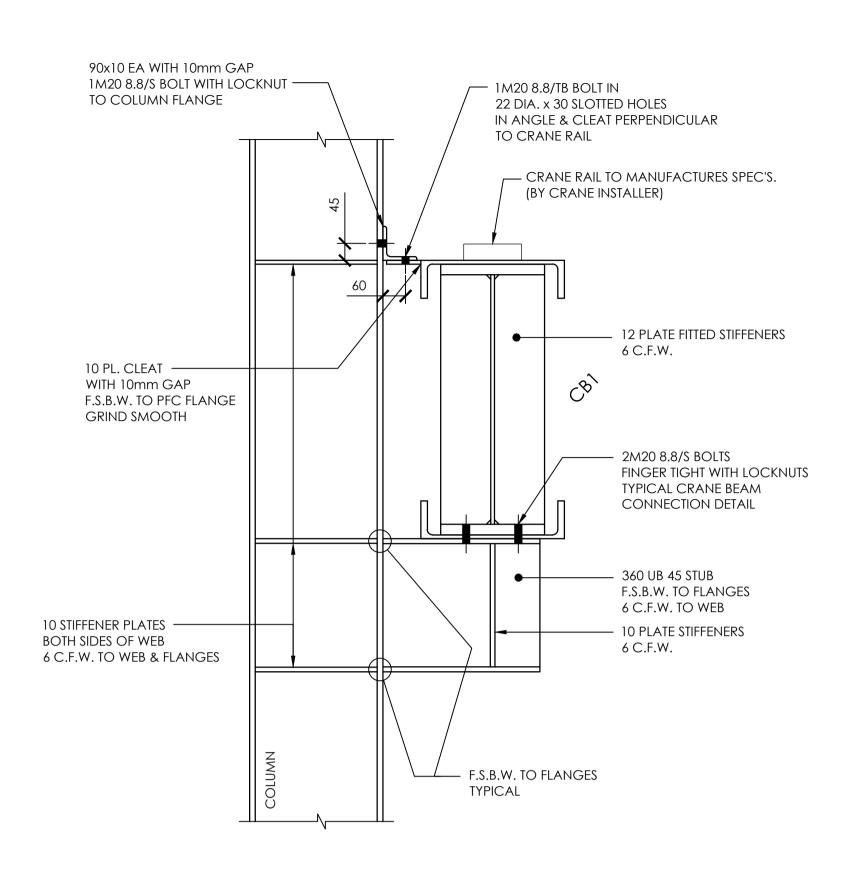
> PROJECT No. TX12805.49 - S4.02 B

DATE ISSUE BY **AMENDMENTS** FOR CONSTRUCTION SUBJECT TO COUNCIL APPROVAL

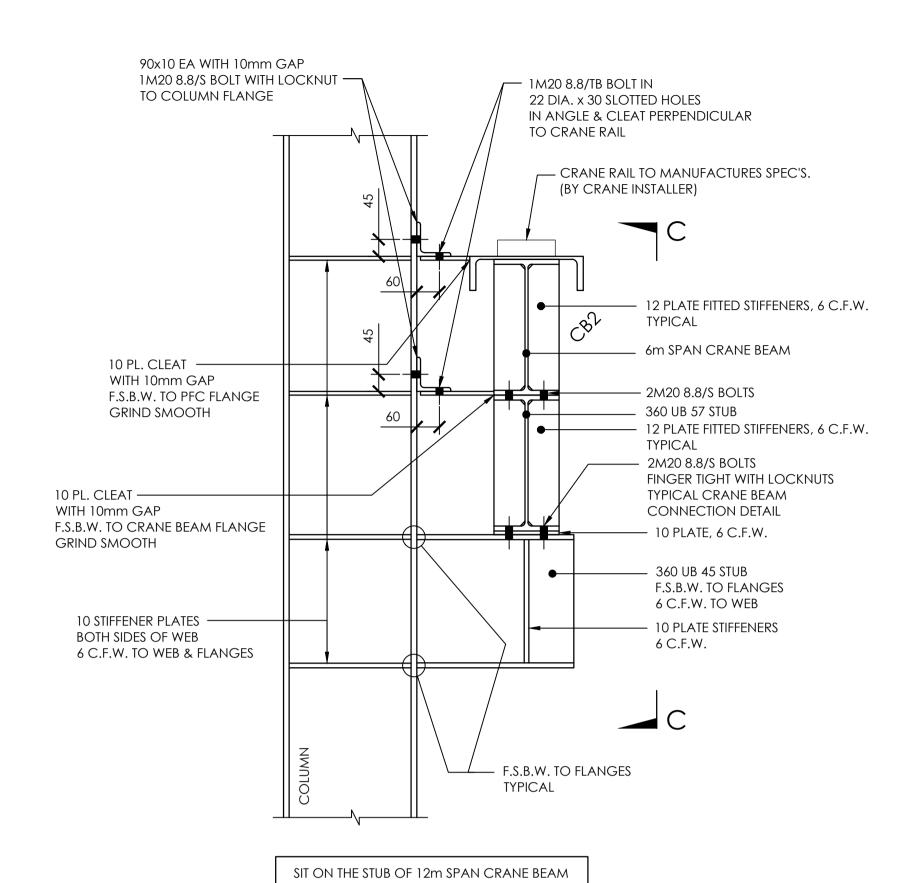
ISSUED FOR APPROVAL

ISSUED FOR APPROVAL

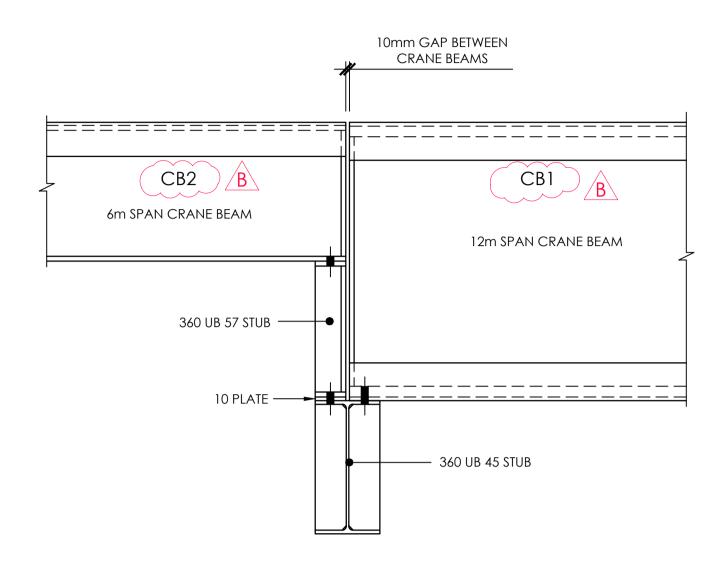
DRAWN DATE S.S. 31.08.20



12m SPAN CRANE BEAM CONNECTION DETAIL SCALE 1:10 AT A1



6m SPAN CRANE BEAM CONNECTION DETAIL SCALE 1:10 AT A1



200 400 600 800 SCALE 1:10 AT A1 SHEET | 1:20 AT A3 SHEET

ISSUED FOR APPROVAL ISSUED FOR APPROVAL

MEGASPAN SHEDS 290 JUBILEE HWY WEST, MT. GAMBIER SA 5290

MALPARA PTY LTD

MALPARA WORKSHOP 291 JUBILEE HIGHWAY WEST MT. GAMBIER SA 5290

S.S. 31.08.20

DESIGNED DRAWN

TX12805.49 - S01



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71 GLEN OSMOND ROAD, EASTWOOD SA 5063 PO BOX 474, TANUNDA SA 5352

DRAWING TITLE STEELWORK DETAILS SHEET 3

PROJECT No. TX12805.49 - S4.03 B SYDNEY | ADELAIDE | DARWIN | PARRAMATTA | MUDGEE | BAROSSA